

Analysis and Design of Residential Building (G+1) using STAAD Pro

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ABSTRACT

The development lately are far than the reach thanks to developing status that our country India holds. With development of country, development of residential buildings takes place. In this paper the planning of residential building is completed with limit state analysis. Limit state method may be a great way to achieve strength of structure with low cost when compare to other design synopsis. The modelling and analysis of the structure is done by using STAAD. Pro 2007, and the designing was done manually. Practical knowledge is an important and vital skill required by every engineer. Then the design follows with different types of loading conditions with different cases of rooms and position of rooms. The Plan is made by AUTOCAD 2018. After plotting the design, analysis is made with the help of STAAD Pro software and the results found out to be same.

KEYWORDS: STAAD Pro, Residential Building, AUTOCAD

How to cite this paper: Ankur Chauhan | Sukrit Jain | Raghav Kumar Tiwary "Analysis and Design of Residential Building (G+1) using STAAD Pro" Published in International Journal of Trend in Scientific Research and Development (ijtsrd), ISSN: 2456-6470, Volume-4 | Issue-6, October 2020, pp.233-243, URL: www.ijtsrd.com/papers/ijtsrd33310.pdf



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I. INTRODUCTION

The basic requirements of human presences are food, apparel's and shelter. From times immemorial man has been attempting endeavors in enhancing their way of life. The purpose of his endeavors has been to give a monetary and productive sanctuary. The ownership of safe house other than being a fundamental, utilized, gives a sentiment security, obligation and demonstrated the societal position of man.

Each individual has an inborn enjoying for a quiet domain required for his charming living, this item is accomplished by having a position of living arranged at the safe and advantageous area, such a spot for agreeable and wonderful living requires considered and kept in stand point.

- A Peaceful domain
- Security from all normal source and atmosphere conditions
- General facilities for group of his neighborhood

The basic requirement for a man is to reduce the cost of construction. The limit state method is use.

II. EXPERIMENTAL SETUP

The main aim of this project is to style a residential building with appropriate reinforcement as per Indian standards with

limit state analysis. The design of residential building takes generation of plan which is completed with the assistance of AUTOCAD software. Before browsing this software the respective positions of rooms (like kitchen, dining hall, master bedrooms, utensils etc). The positioning of rooms is done with respect to aspects of building.

A. Arrangements of Rooms

1. Aspects

Aspect means particular arrangement of doors and windows in external walls of residential building while environment to undergo it. The important aspect in panning is not only providing the sunshine but also hygiene and eco-friendly environment. The room is based upon the allowance of air and light and referred to such particular aspect. As per the plan the different arrangements of room are shown below.

ROOM(IN BOTH FLOORS)	ASPECT
ENTRANCE	NORTH
KITCHEN	EAST
DINING HALL	SOUTH
LIVING ROOM	SOUTH WEST
PRAYER ROOM	EAST
MASTER BEDROOM	SOUTH EAST
STAIRCASE	NORTH
VERANDAH	WEST OR SOUTH WEST
UTENSILS	WEST NEAR TO BEDROOM
BEDROOM	WEST

TABLE: 1

ROOM	DIMENSIONS
GROUND FLOOR	
KITCHEN	3x2.5
DINING HALL	3x2.5
MASTER BEDROOM	3.5x3
BEDROOM	4x3.5
LIVING ROOM	5x3
PRAYER ROOM	1x1.5
UTENSILS	2x1.5
FIRST FLOOR	
KITCHEN	3x2.5
DINING HALL	3x2.5
MASTER BEDROOM	3.5x3
BEDROOM	4x3.5
PRAYER ROOM	1.5x2
UTENSILS	1.8x2

TABLE: 2

2. Size

The total area of residential building is 198.00 sq m. The area is divided into number of rooms as per requirement. In keeping the view of health and ventilation, The sizes of room are provided keeping in view of National Building code, the different dimensions of rooms are provided as.

The plan section and elevation of residential building which are drafted in AUTOCAD software in 2018 version and is shown below.

4. GROUND FLOOR

3. Plan

The residential building consists of two storeys. First storey is referred as ground floor and second is referred as first floor. The respective plan for ground floor, first floor upto top floor which are drafted in AUTOCAD software are shown as individually as below.

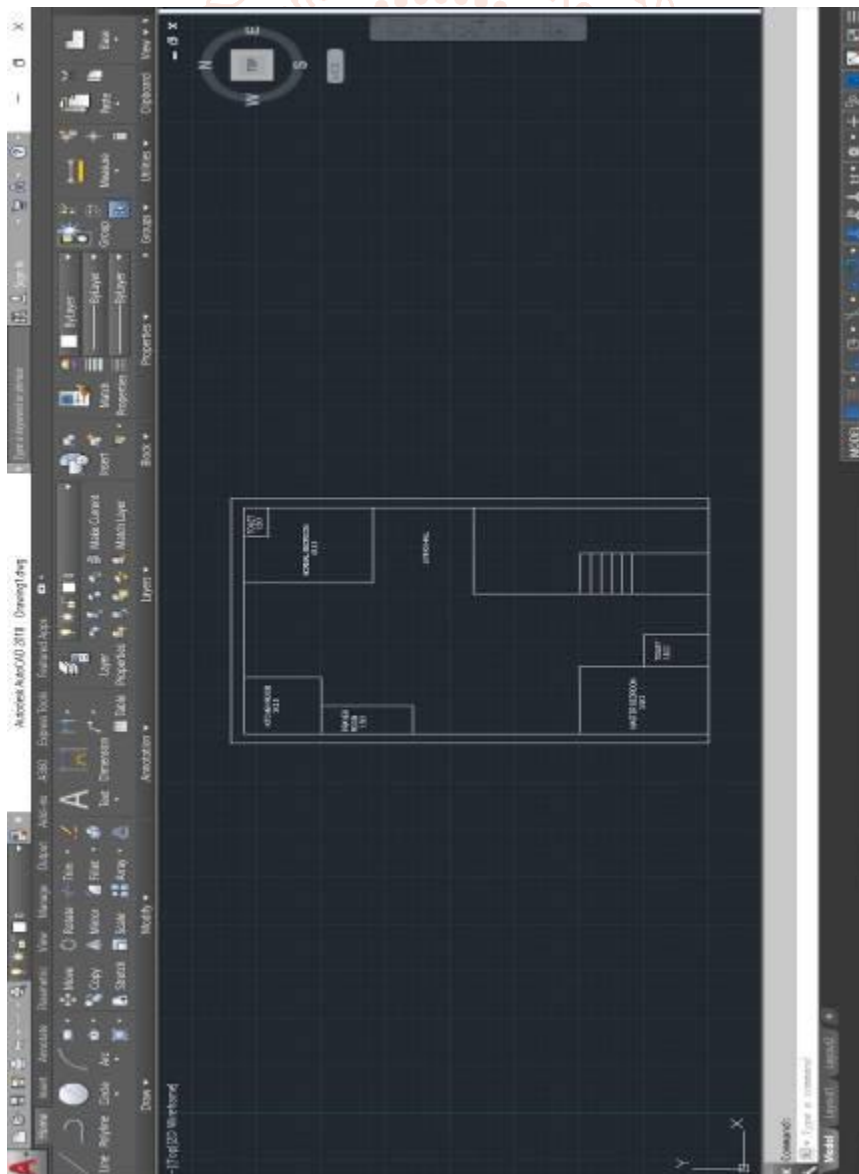


FIG.1

5. FIRST FLOOR

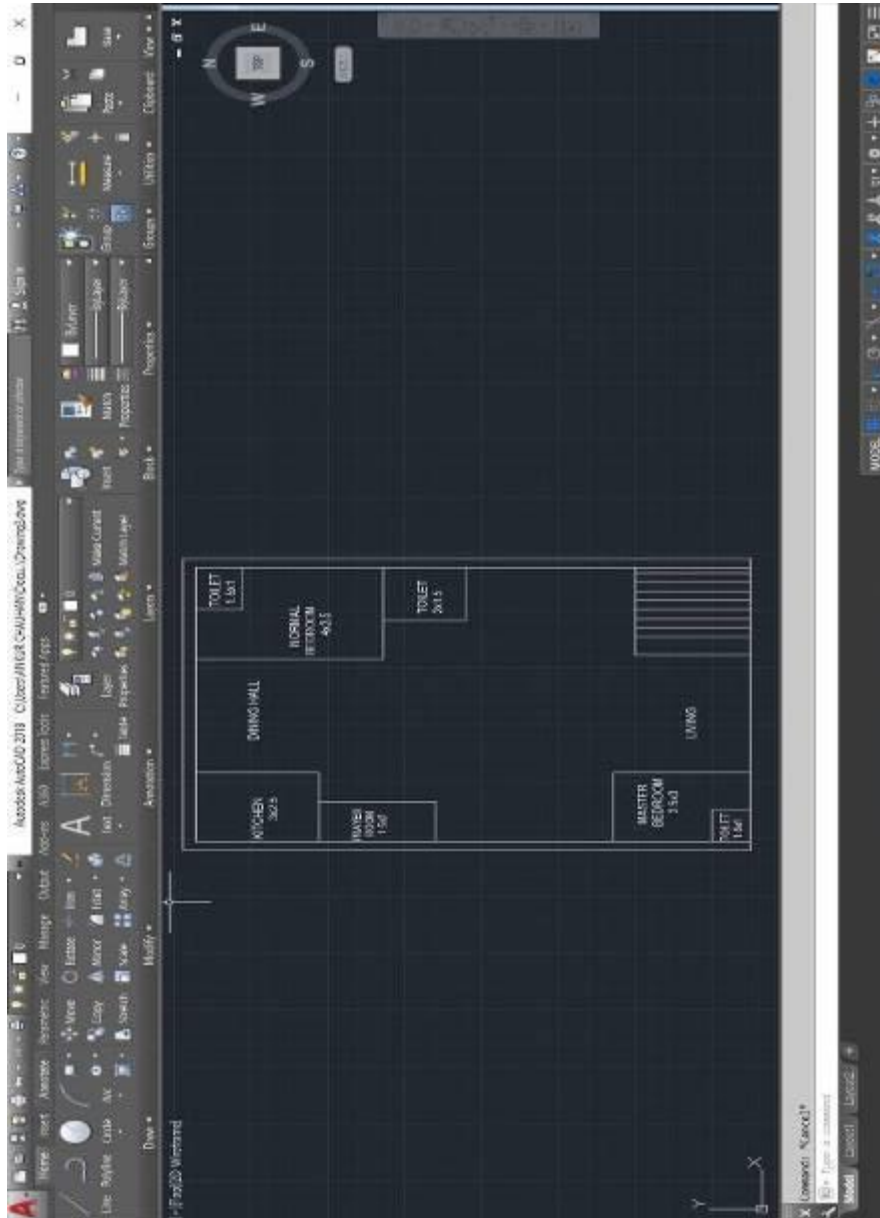


FIG.2

III. DESIGN OF RESIDENTIAL BUILDING

The design of residential building is carried out as per Limit state analysis or Limit state method. The IS codes used in the design are IS:456 2000 and IS:875 1980

IV. DESIGN OF SLAB

The foremost important point in design of slab is analysis of loads. The loads are directly provided in Indian Standard IS: 875 1980 (Part 1 for dead load; Part 2 for live load; Part 3 for wind load)

As per IS: 875 1980 part II, the live loads for different types of rooms rested on ground floor is sort out as.

ROOM	LOADS AS PER IS:875 1980
KITCHEN	3KN/m ²
DINING HALL	4 KN/m ²
MASTER BEDROOM	3 KN/m ²
BEDROOM	2 KN/m ²
LIVING ROOM	2 KN/m ²
PRAYER ROOM	2 KN/m ²
UTENSILS	2 KN/m ²

TABLE: 3

A. Kitchen

The dimensions of kitchen room are 3x2.5 m (as we discussed earlier)

1. Type of Slab

The type of slab is decided based upon ratio of longer span to shorter span

$$\text{Longer span/ shorter span} = 3/2.5 = 1.2$$

If this ratio is less than 2 then two way slab (If more than 2 one way slab)

As per our dimensions of kitchen the design goes with two way slab

The super imposed load for kitchen is taken as 3 KN/m².

2. Depth of slab

Based upon the stiffness, selected l/d ratio as 28

$$\text{i.e., } 25000/d = 28$$

$$d = 89.28 \text{ mm (approx. } 90 \text{ ,,)}$$

Provide 30 mm effective cover

Over all depth of slab = 120 mm

3. Loads per meter

$$\text{Dead load} = 25 \times 1 \times 0.12 = 3 \text{ KN/m}$$

$$\text{Super imposed load} = 3 \text{ KN/m}$$

$$\text{Total load} = 6 \text{ KN/m}$$

$$\text{Factored load} = 1.5 \times 6 = 9 \text{ KN/m}$$

4. Effective span

As per IS: 456 2000 clause 22.2

$$\text{The effective span is given as } l_{\text{eff}} = 90 + 2500 = 2590 \text{ mm}$$

5. Moments

As per IS: 456 2000 table 27

$$M_x = \alpha_x W l_{\text{eff}}^2$$

$$\alpha_x = 0.084$$

$$M_x = 0.084 \times 9 \times 2.59 \times 2.59 = 5.07 \text{ KNm}$$

$$M_y = \alpha_y W l_{\text{eff}}^2$$

$$\alpha_y = 0.059$$

$$M_y = 0.059 \times 9 \times 2.59 \times 2.59 = 3.56 \text{ KNm}$$

6. Check for depth

$$M = 0.138 f_{ck} b d^2$$

$$5.07 \times 10^6 = 0.138 \times 20 \times 1000 \times d^2$$

$$D = 42.85 \text{ mm (HENCE SAFE)}$$

7. Reinforcement**8. Longer span**

$$M_x = 0.87 f_y A_{st} d (1 - f_y A_{st} / f_{ck} b d)$$

The area of reinforcement for shorter span is obtained as

$$A_{st} = 112.47 \text{ mm}^2$$

9. Spacing

Provide 12 mm \emptyset diameter bars

$$\text{Spacing, } S = a_{st} / A_{st} \times 1000$$

$$a_{st} = \pi \times 12^2 / 4$$

$$\text{We get spacing as } S = 1005.57 \text{ mm}$$

As per IS: 456 2000, Clause 26.3.3

Spacing,

$$= 3d = 270 \text{ mm}$$

$$= 300 \text{ mm}$$

Whichever is less

Provide 12 mm \emptyset bars @ 270 mm c/c

10. Shorter span

$$M_y = 0.87 f_y A_{st} d (1 - f_y A_{st} / f_{ck} b d)$$

The area of reinforcement for shorter span is obtained as

$$A_{st} = 162.08 \text{ mm}^2$$

11. Spacing

Provide 12 mm \emptyset diameter bars

$$\text{Spacing, } S = a_{st} / A_{st} \times 1000$$

$$a_{st} = \pi \times 12^2 / 4$$

$$\text{We get spacing as } S = 697.78 \text{ mm}$$

As per IS: 456 2000, Clause 26.3.3

Spacing,

$$= 3d = 270 \text{ mm}$$

$$= 300 \text{ mm}$$

Provide 12 mm \emptyset bars @ 270 mm c/c

12. Edge Strip

As per IS: 456, Clause 26.5.1.2.1

$$A_{st} = 0.12 \% \text{ of gross area} = 0.12 \times 1000 \times 120 / 100 = 144 \text{ mm}^2$$

13. Spacing

Provide 8 mm \emptyset diameter bars

$$\text{Spacing, } S = a_{st} / A_{st} \times 1000$$

$$a_{st} = \pi \times 8^2 / 4$$

$$\text{We get spacing as } S = 349.06.78 \text{ mm}$$

As per IS: 456 2000, Clause 26.3.3

Spacing,

$$= 5d = 450 \text{ mm}$$

$$= 450 \text{ mm}$$

Provide 8 mm \emptyset bars @ 350 mm c/c

14. Check for deflection

$$l/d = 20$$

$$f_s = 0.58 f_y A_{st}(\text{req}) / A_{st}(\text{pro})$$

As per IS: 456 2000, Clause 23.2.1

Modification factor = 2

$$l/d(\text{req}) = 40$$

$$l/d(\text{get}) = 2590/90 = 28.78 \text{ (HENCE SAFE)}$$

B. Design of Beam

$$D = 300 \text{ mm}$$

$$d = 250 \text{ mm}$$

$$d' = 50 \text{ mm}$$

$$b = 125 \text{ mm}$$

$$f_{ck} = 20 \text{ N/mm}^2$$

$$f_y = 415 \text{ N/mm}^2$$

1. Loads per meter

$$\text{Dead load} = 25 \times 0.25 \times 0.125 = 0.781 \text{ KN/M}$$

$$\text{Super imposed load} = 2 \text{ KN/M}$$

$$\text{Live load} = 10.78 \text{ KN/M}$$

$$\text{Total load} = 13.56 \text{ KN/M}$$

$$\text{Factor load} = 1.5 \times 13.56$$

$$W_u = 20.34 \text{ KN/M}$$

$$\text{Bending moment} = w_u l^2 / 8$$

$$= 20.34 \times 3.52^2 / 8$$

$$M_u = 30.145 \text{ KNm}$$

Check for depth:-

$$M_u = 0.138 f_{ck} b d^2$$

$$= 0.138 \times 20 \times 125 \times d^2$$

$$d = 295.59 \text{ mm}$$

Depth is failed (So provide double reinforcement)

2. Reinforcement

3. Ast in compression

$0.87f_y A_{st1} = 0.36f_{ck} b x_{umax}$
 $A_{st1} = 0.36 \times 20 \times 125 \times 120 / 0.87 \times 415$
 $A_{st1} = 299.12 \text{ mm}^2$
 (From sp 16 charts) $F_{sc} = 342.4 \text{ N/mm}^2$
 μ limit $= 0.138 f_{ck} b d^2$
 μ limit $= 21.56 \text{ KNM}$
 $\mu_2 = 8.585 \text{ KNM}$
 $\mu_2 = f_{sc} A_{sc} (d-d')$
 $8.58 \times 106 = 342.4 \times A_{sc} \times (250-50)$
 $A_{sc} = 125.36 \text{ mm}^2$

4. Additional tensile steel (Ast2)

$0.87f_y A_{st2} = f_{sc} A_{sc}$
 $A_{st2} = f_{sc} A_{sc} / 0.87f_y$
 $A_{st2} = 118.88 \text{ mm}^2$

5. Total tensile steel

$A_{st} = A_{st1} + A_{st2}$
 $A_{st} = 418 \text{ mm}^2$

6. No. of bars (in tension)

$A_{st} = 418, \phi = 18 \text{ mm}$
 $= 418 / \pi / 4 \times 18^2$
 $= 2 \text{ bars}$

7. In compression

$\Phi = 12 \text{ mm}$
 $= 125.36 / \pi / 4 \times 12^2$
 $= 2 \text{ bars}$
 Provide 2-18mm bars in tension
 Provide 2-12mm bars in compression

8. Column (Axial)

Height $= 2.75 \text{ m}$
 Factor load $= 1100 \text{ KN}$
 Let Assume the Gross area % of steel
 $A_{sc} = 1\% A_g = 0.01 A_g$
 Area of concrete, $A_c = A_g - A_{sc}$
 $A_g - 0.01 A_g = 0.99 A_g$
 Axially load column:-
 $P_u = 0.4 f_{ck} A_s + 0.67 f_y A_{st}$
 $1100 \times 103 = 0.4 \times 20 \times 0.99 A_g + 0.67 \times 415 \times 0.01 A_g$
 $A_g = 102798.93 \text{ mm}^2$
 $S_2 = 102798.9$
 $S = 320 \times 320 \text{ mm}$
 $A_{sc} = 0.01 \times 102798.9$
 $A_{sc} = 1027.989 \text{ mm}^2$
 Provide 4bars of 18mm diameter

9. Lateral reinforcement

From IS:456-2000, clause-26.5.3.2

10. Ties

Adopt 6mm ϕ bars

11. Pitch

320mm (or)
 $16 \times \phi = 16 \times 18 = 288 \text{ mm}$ (or)
 300mm
 Provide 6mm lateral ties @288mm/c

C. Design of Footing

Axial load $= 1100 \text{ KN}$
 Size of column $= 320 \times 320 \text{ mm}$

Soil bearing capacity $= 200 \text{ KN/M}^2$
 $F_{ck} = 20 \text{ N/MM}^2, F_y = 415 \text{ N/MM}^2$

1. Size of footings

Assume dead load 10%
 Dead load $= 1.1 \times 733.33 / 200$
 Area $= 4 \text{ m}^2$
 $S_2 = 4$
 $S = 2 \times 2 \text{ m}$

2. Upward soil pressure

$P_u = 733.33 \times 1.5$
 $P_u = 1100 \text{ KN}$
 $q_u = 1100 / 2 \times 2 = 275$
 $q_u = 0.275 \text{ N/mm}^2$

3. Moment

$\mu = q_u x B / 8 (B-b)^2$
 $0.275 \times 2000 / 8 \times (2000-320)^2$
 $\mu = 194.04 \text{ KNM}$

4. Depth

$\mu = 0.138 f_{ck} b d^2$
 $d = 400 \text{ mm}$

5. Reinforcement

$\mu = 0.87 f_y A_{st} d (1 - f_y A_{st} / f_{ck} b d)$
 $A_{st} = 1393.98 \text{ mm}^2$

6. Spacing

From IS:456-2000, Clause-26.3.3
 $\Phi = 12 \text{ mm}$
 $S = a_{st} / A_{st} \times B$
 $S = 162.26 \text{ mm}$
 Provide 12mm ϕ @ 160mm/c
 One-way shear:-
 $V_u = q_u B [(B-b) / 2 - d]$
 $V_u = 242000 \text{ KN}$

7. Nominal shear stress

$\tau_v = V_u / b d$
 $\tau_v = 0.3025 \text{ N/MM}^2$
 From IS:456-2000, Table-19
 $\%P = 100 A_{st} / b d$
 $\%P = 0.17$
 By linear interpolation:
 $\tau_{c1} = 0.296 \text{ N/mm}^2$
 $\tau_v > \tau_{c1}$ (Provide shear reinforcement)

8. Needed reinforcement

9. Design stirrups

2legged-6mm
 $A_{sv} = 56.54 \text{ mm}^2$
 $V_{us} = V_u - \tau_c b d$
 $V_{us} = 5200 \text{ KN}$
 Vertical stirrups:-
 $V_{us} = 0.87 f_y A_{sv} d / S_v$
 $S_v = 1570.28 \text{ mm}$

10. Check

From IS:456-2000, Clause 26.5.1.5
 $0.75 d = 300 \text{ mm}$ (or)
 300mm
 Provide 2legged-6mm ϕ @300mm/c

Two-way shear:-
 $V_u = 2 \times q_u (B - (b + d))$
 $V_u = 957440 \text{ KN}$
 $A = 4(b + d)d$
 $A = 1152 \times 103$
 $\tau_v = 957440 / 1152 \times 103$
 $\tau_v = 0.83 \text{ N/mm}^2$
 $\tau_c = 0.25 f_{ck}$
 $\tau_c = 1.11 \text{ N/mm}^2$
 $\tau_v < \tau_c$ (Hence Safe)

$d = 190 \text{ mm}$

5. Tension reinforcement

$M_u = 0.87 f_y A_{st} d (1 - f_y A_{st} / f_{ck} b d)$
 $A_{st} = 812 \text{ mm}^2$

6. Spacing

$S = a_{st} / A_{st} \times B$
 $S = 139.3 \text{ mm}$
 Hence provide 12mm bars @ 130mm/c

7. Distribution steel

$A_{st} = 0.12\%$ of gross area
 $= 0.12 \times 1000 \times 220 / 100 = 264 \text{ mm}^2$
 Using 8mm bars, spacing
 $S = \pi / 4 \times 8^2 = 190.4 \text{ mm}$
 Hence provide 8mm bars @ 190mm c/c

D. Design of Stair Case

1. Proportioning of Stairs

Dimension of stair hall = 2.5x4.5m
 Height of floor = 3.3m
 Rise R = 225mm
 Tread T = 150mm

2. Effective span

From IS:456-2000, Clause-22.2
 $l_e = 4.73 \text{ m}$
 Thickness of waist slab:
 Assume depth = 2730/25 = 189.2mm
 $d = 190 \text{ mm}, D = 220 \text{ mm}$

3. Loads

Weight of waist slab = $D \sqrt{(1 + (R/T))} \times 25 = 6.61 \text{ KN/m}$
 Weight of steps = $(1/2RT) \times 25 = 1.875 \text{ KN/m}^2$
 Live load = 3KN/m²
 Floor finish = 0.6KN/m²
 Total load = 12.1KN/m²
 Factor load (w_u) = 1.5x12.1 = 18.15KN/m²
 Bending moment $M_u = w_u l^2 / 8$
 $M_u = 50.76 \text{ KNM}$

4. Depth

$M_u = 0.138 f_{ck} b d^2$

8. Reinforcement details

- a. Beams
 Provide 2-18mm bars in tension
 Provide 2-12mm bars in compression
- b. Column
 Provide 4 bars of 18mm diameter
 Provide 6mm lateral ties @ 288mm c/c
- c. Footing
 Provide 12mmφ @ 160mm/c
 Provide 2legged-6mmφ @ 300mm/c
- d. Slab
 Provide 12mmφ @ 270mm/c
 Provide 12mmφ @ 280mm/c
 Provide 8mmφ @ 350mm/c

9. Reinforcement sketches

10. Slab

11. Kitchen room

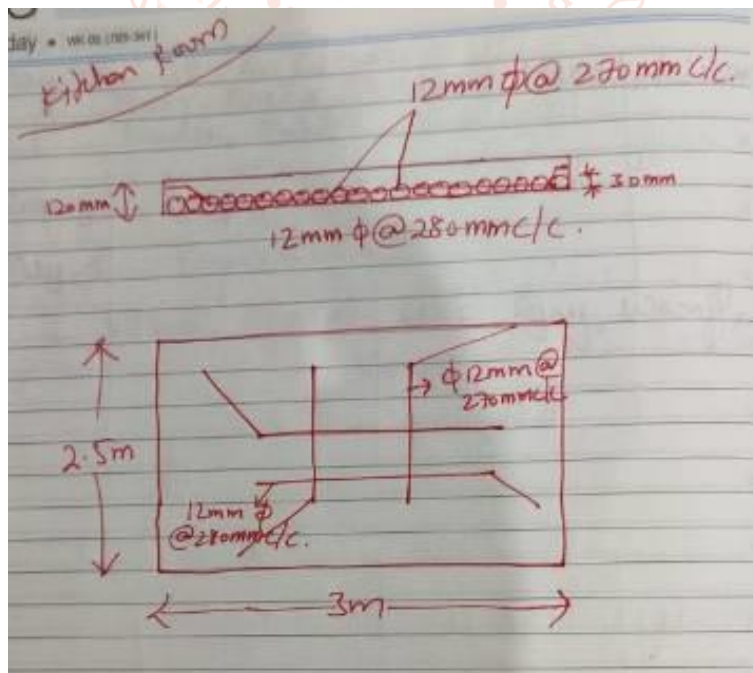


FIG.3

12. Other Rooms

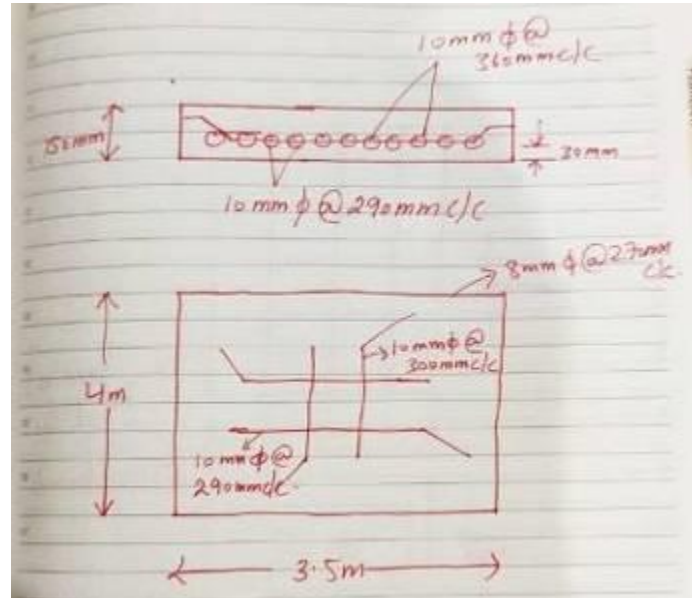


FIG.4

13. Columns

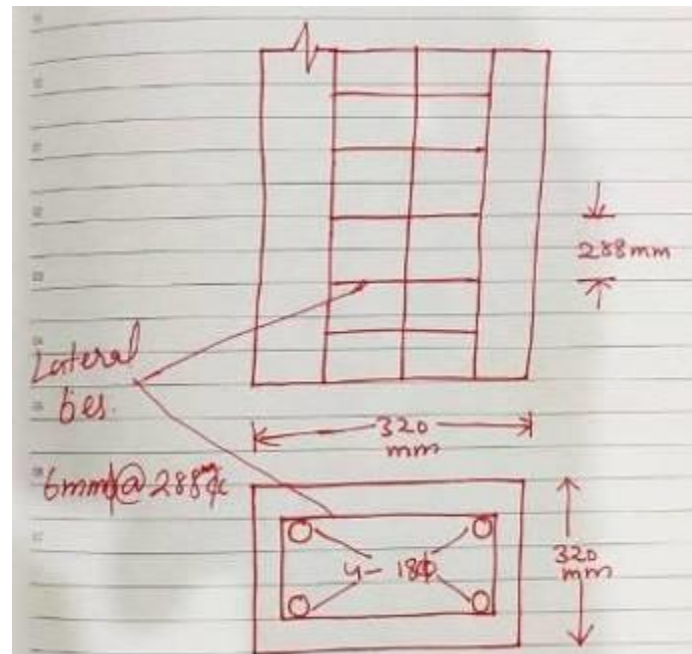


FIG.5

14. Footings

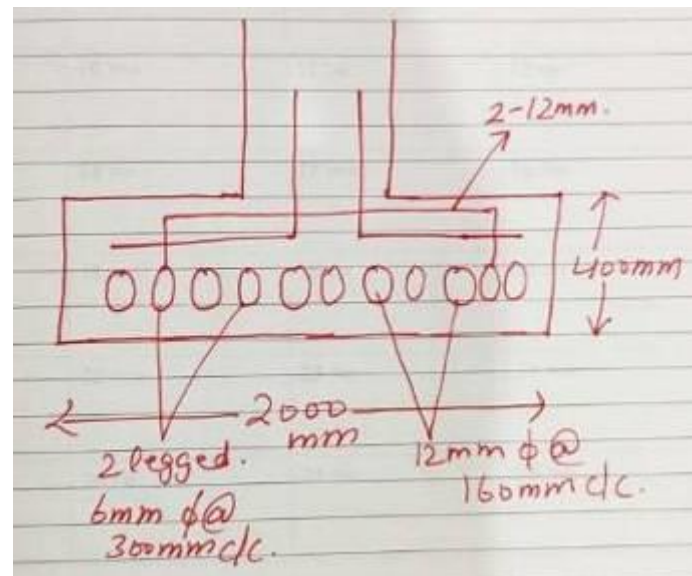


FIG.6

15. Stair cases

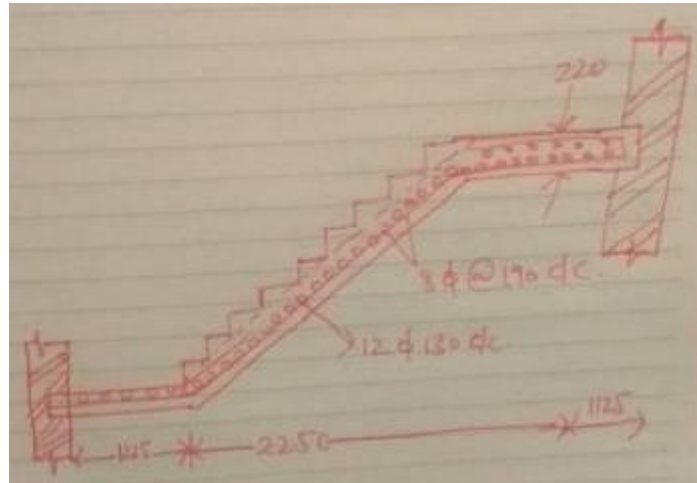


FIG.7

16. Results

The results shown are based on analysis of designed model in STAAD Pro software. The model generated in STAAD Pro is shown in figure below as position of beams and column.

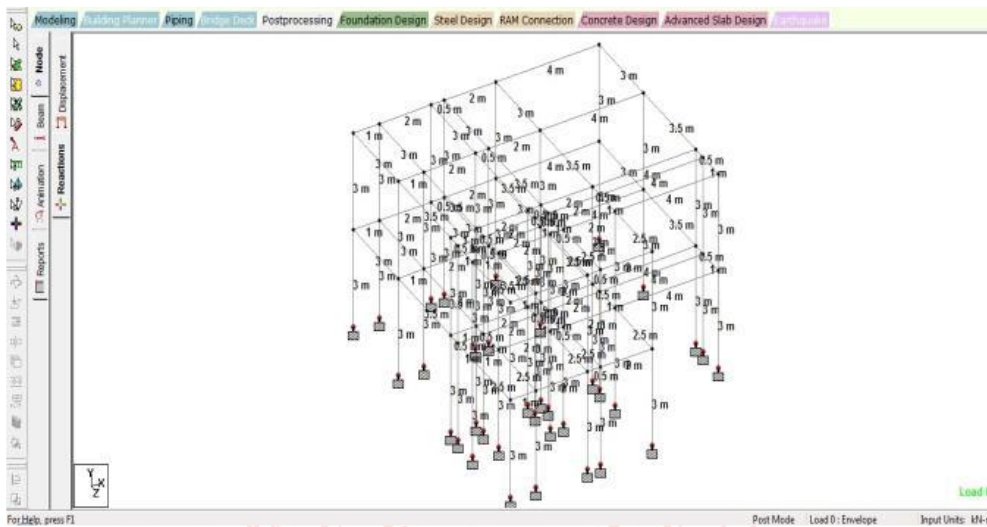


FIG.8

17. Forces

The analysis of buildings start with analysis of forces in each member in their respective axis, then moments, stresses and eventually deflection. First of all the forces are found in X, Y and Z- direction and there diagram are shown below.

18. Axial Forces in X- direction

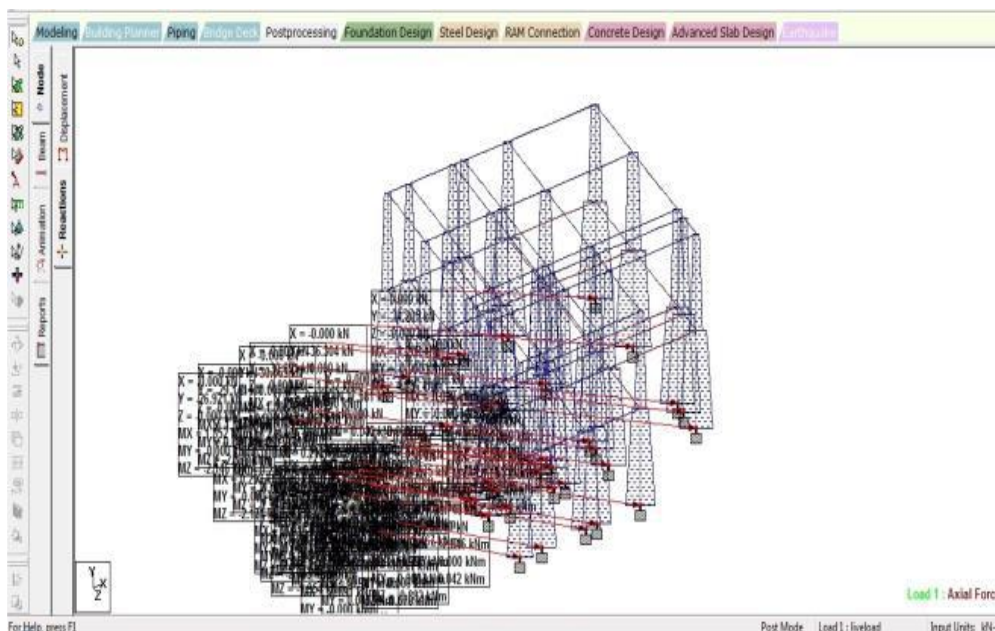


FIG.9

19. Axial Forces in Y-direction

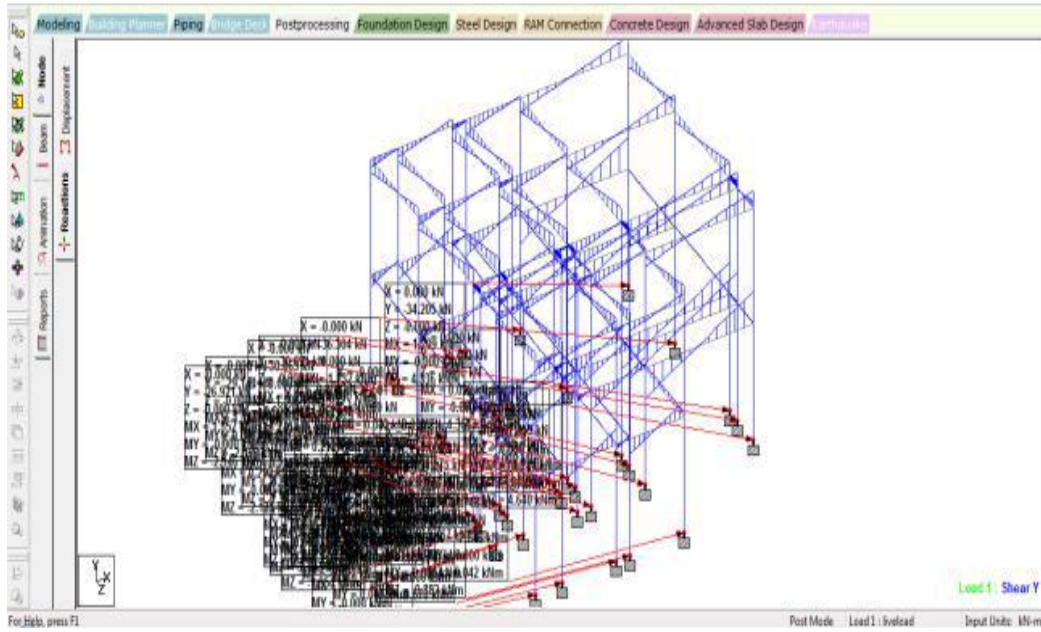


FIG.10

20. Axial Forces in Z-direction

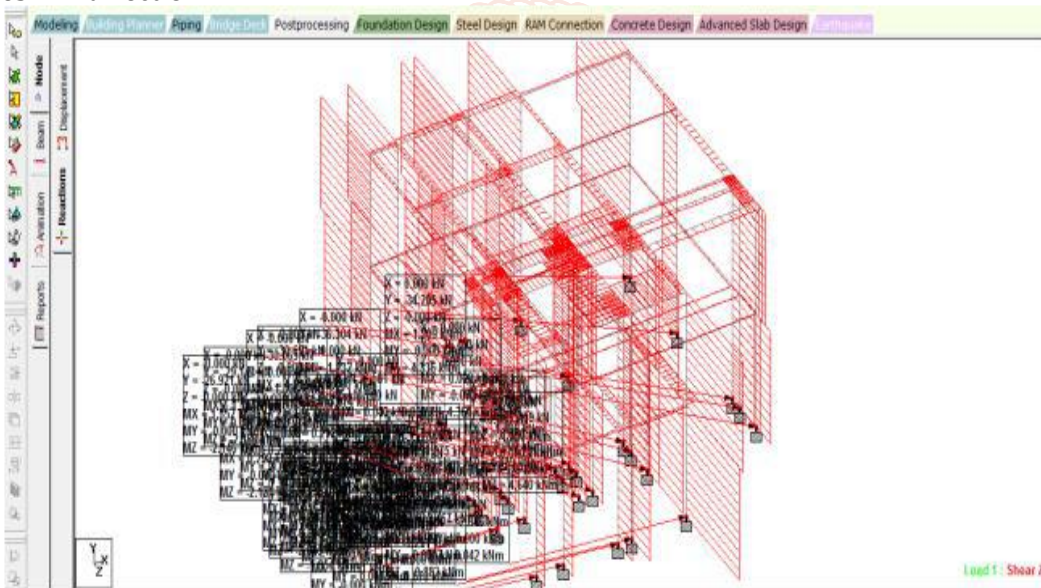


FIG.11

21. The Torsion in X-direction

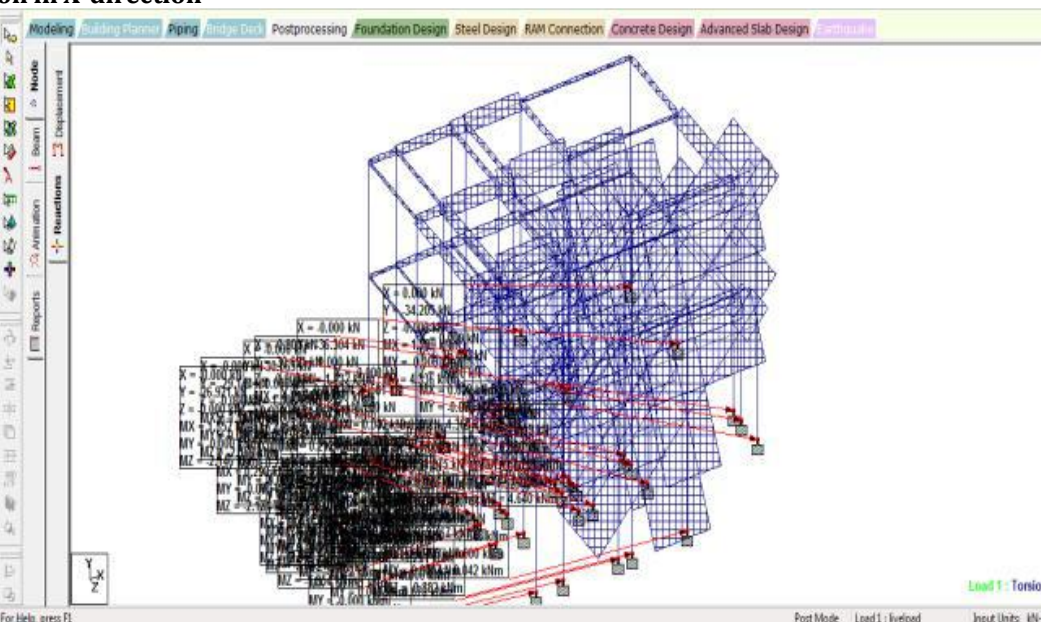


FIG.12

22. The Moment in Y-direction

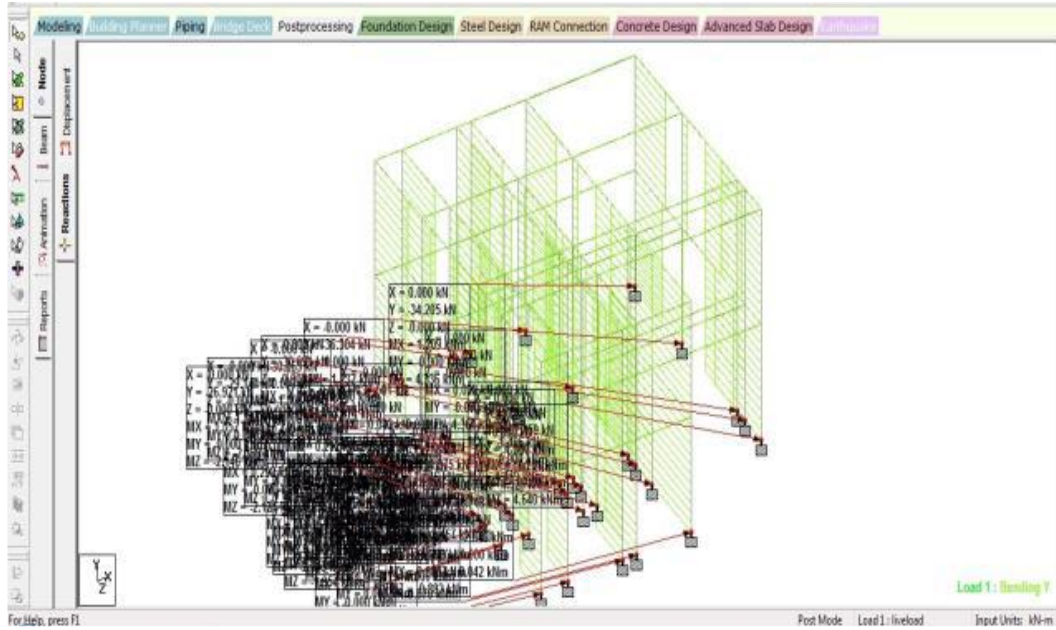


FIG.13

23. The Moment in Z-direction

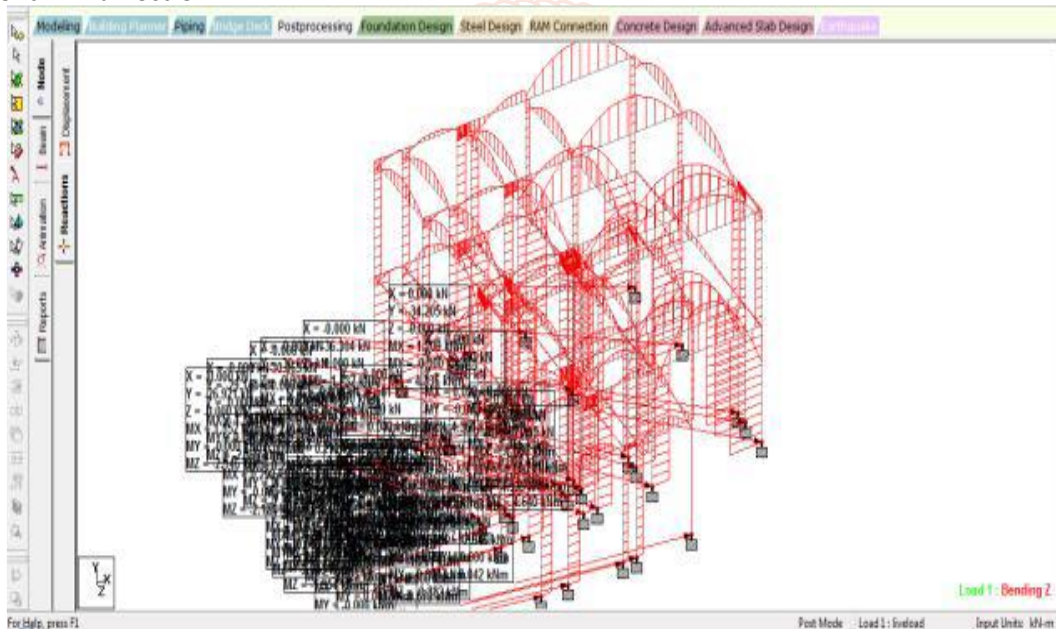


FIG.14

24. Stresses

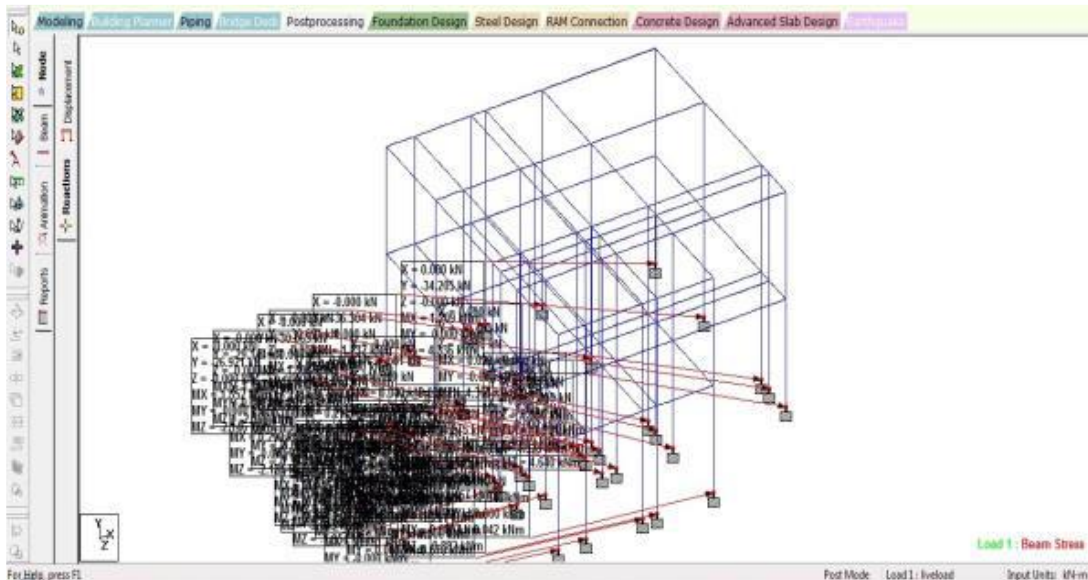


FIG.15

25. Deflection

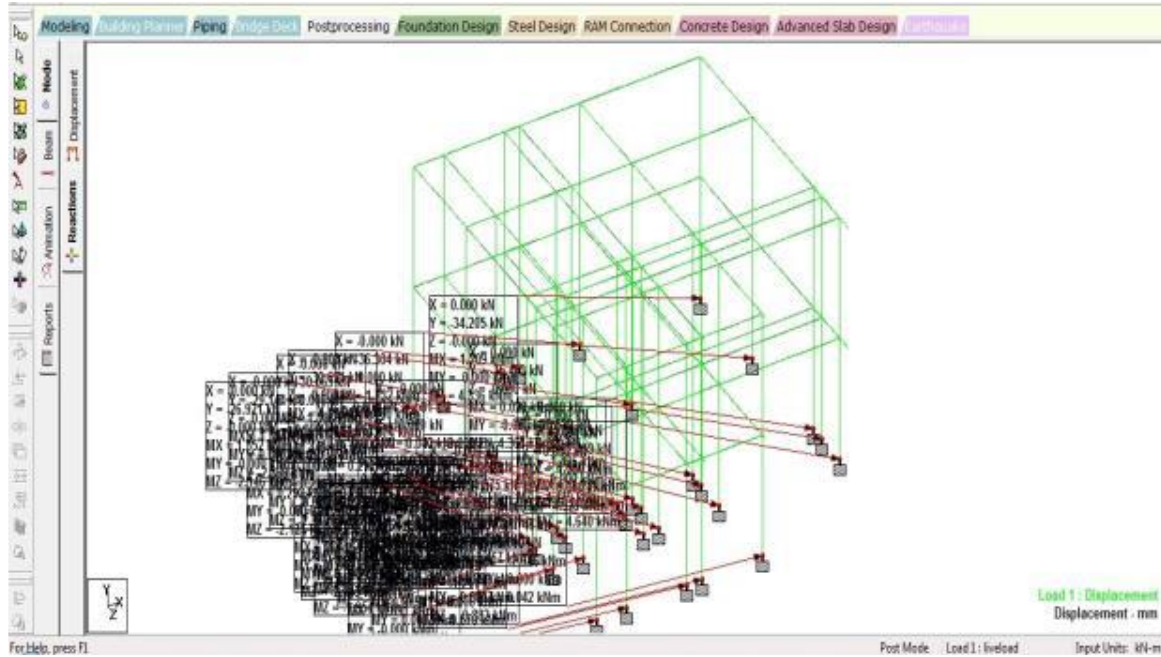


FIG.16

26. Final static report

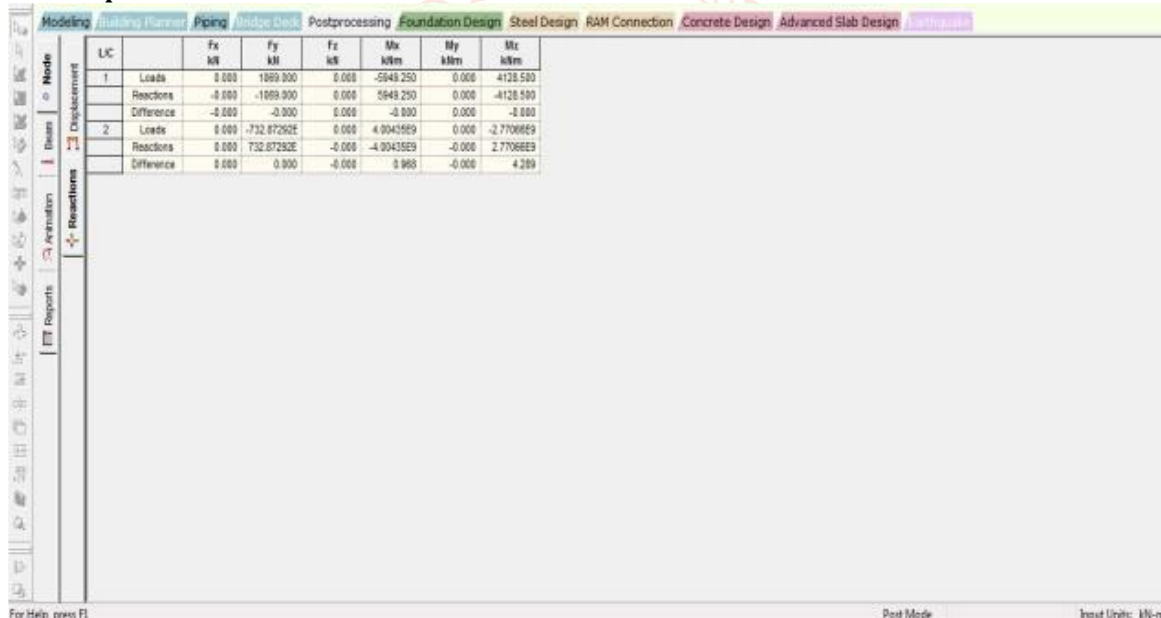


FIG.17

The final static report is given as
Hence all the loads and displacements are resisted by structure itself

V. CONCLUSION

The method used is limit state analysis, the factor of safety for concrete is 1.5 and steel is 1.1 it means 50% more concrete and 10% more steel is consider. In working state method which is broadly followed in our country has factor of safety of 3 for concrete and 1.7 for steel it means 200% more concrete and 70% more steel.

The amount of more concrete and steel, bigger areas can be seen in working stress method. As we can reduce out area by following limit state method and hence also proved as economical.

The design review the study of AUTOCAD 2018 and analysis with STAAD pro and found out the structure is safe in deflections, stresses, loads and moments.

The aspects and prospects are made according to NBC of India, which gives various advantages over random arrangements.

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